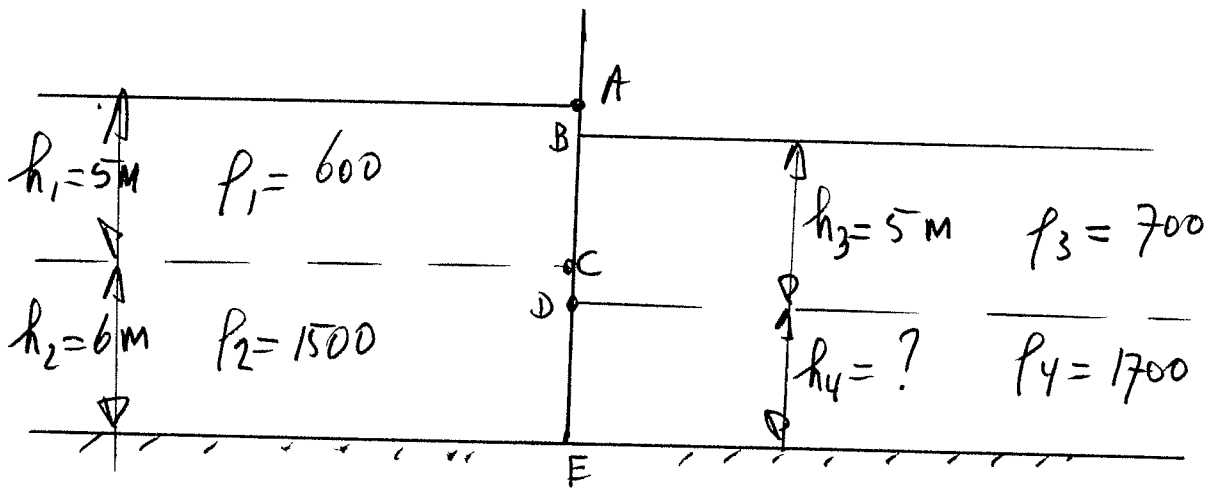


Examination Fluid Mechanics I (GEO3-4307) November 2009

1. Left and right of a gate are two layers of fluid ($g = 10 \text{ m/s}^2$):

$\rho_1 = 600 \text{ kg/m}^3,$	$h_1 = 5 \text{ m}$
$\rho_2 = 1500 \text{ kg/m}^3,$	$h_2 = 6 \text{ m}$
$\rho_3 = 700 \text{ kg/m}^3,$	$h_3 = 5 \text{ m}$
$\rho_4 = 1700 \text{ kg/m}^3,$	$h_4 = ?$

- a) compute the water depth h_4 at which the fluid pressure at the bottom on both sides of the gate is equal.
- b) compute the resulting pressures at all interfaces (points A, B, C, D, and E) and make a plot of the pressure distribution.
- c) compute the total resulting horizontal fluid force at the gate.



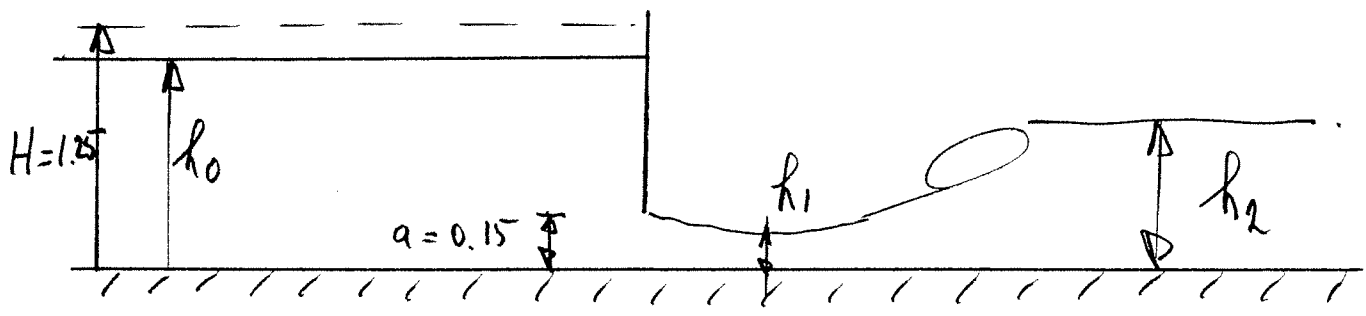
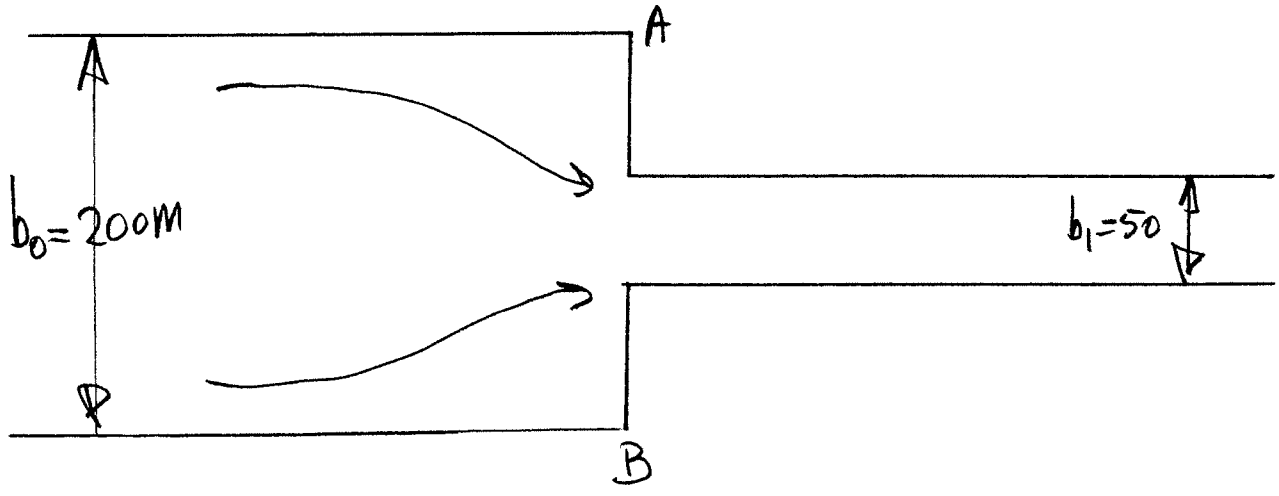
2 A channel with width $b_0 = 200$ m changes into a structure with a gate. The width of the gate is equal to the width $b_1 = 50$ m of the downstream channel.

Other data:

- $H_0 =$ energy height upstream = 1.25 m
- $a =$ opening of gate = 0.15 m
- $\mu =$ contraction coefficient = 0.66
- $g =$ 10 m/s²

- a. Make a plot of the energy line along the total traject
- b. Compute water depth h_1
- c. Compute discharge Q
- d. Compute the water depth h_2
- e. Compute the water depth h_0
- f. Compute the force at the wall AB (including the gate)

Plan view (plattegrond)



side view (zij-aanzicht)

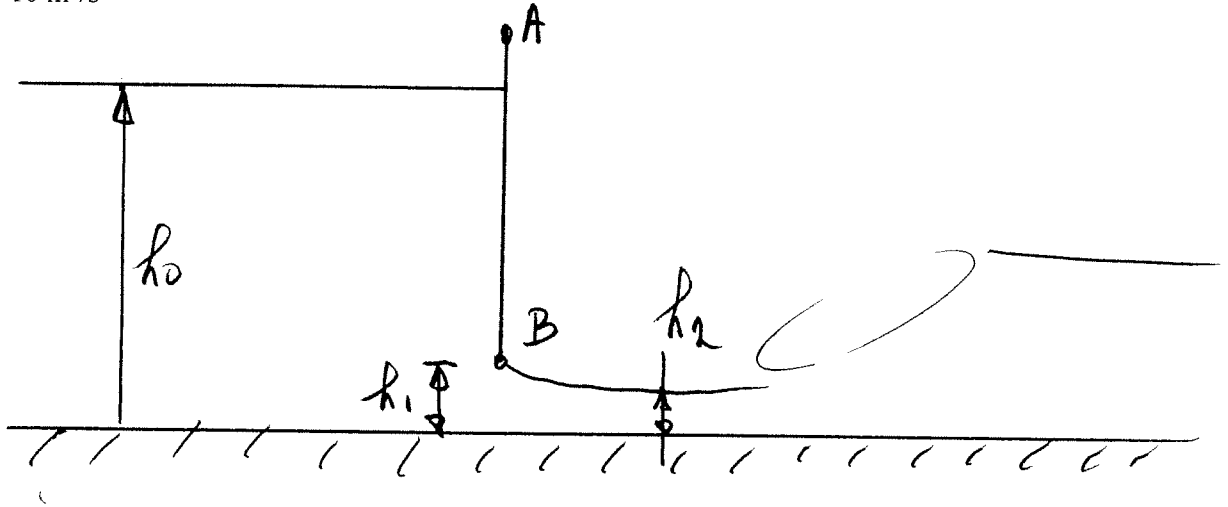
3. A channel with a horizontal bottom has a gate AB at the end with opening h_1 . The velocity head upstream of the gate is measured by a Pitot-tube and is 0.02 m.

Given are:

h_0 = water depth upstream = 1 m

μ = contraction coefficient = 0.6

$g = 10 \text{ m}^2/\text{s}$



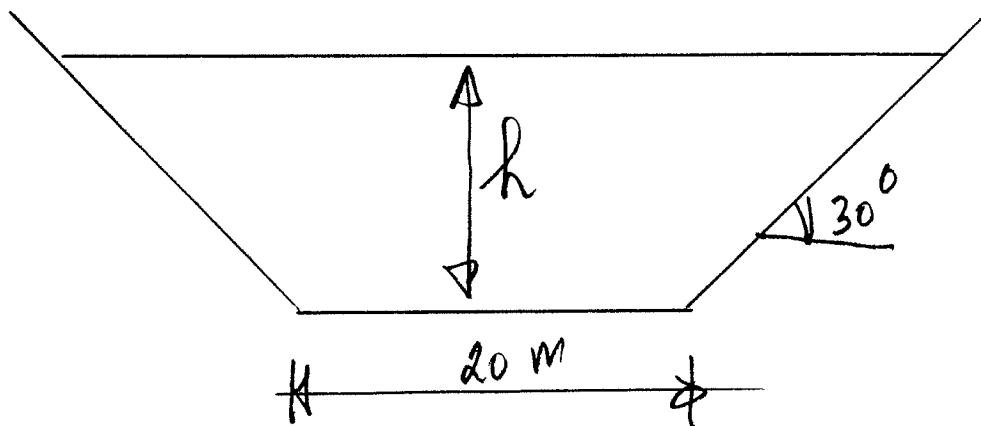
- what is the velocity v_0 upstream of the gate and what is the discharge q ?
- what is the depth h_2 and what is the depth h_1 under the gate; what is the Froude number at point 2
- compute the force at the gate?
- The pressures along the gate are measured in 7 points

$z_1 = 0.1 \text{ m}$	$p_1 = \text{hydrostatic}$
$z_2 = 0.2 \text{ m}$	$p_2 = \text{hydrostatic}$
$z_3 = 0.3 \text{ m}$	$p_3 = \text{hydrostatic}$
$z_4 = 0.4 \text{ m}$	$p_4 = 3300 \text{ N/m}^2$
$z_5 = 0.5 \text{ m}$	$p_5 = 3700 \text{ N/m}^2$
$z_6 = 0.6 \text{ m}$	$p_6 = 3900 \text{ N/m}^2$
$z_7 = 0.7 \text{ m}$	$p_7 = 3400 \text{ N/m}^2$

What is the force at the gate based on the pressure measurements?

What is the hydrostatic force at the gate?

4. A uniform river has a trapezoidal cross-section with
 $h = 4.5 \text{ m}$, $I = 0.0001$, $B = 20 \text{ m}$, $g = 10 \text{ m/s}^2$, side slope angle = 30 degrees,
viscosity = $0.00001 \text{ m}^2/\text{s}$

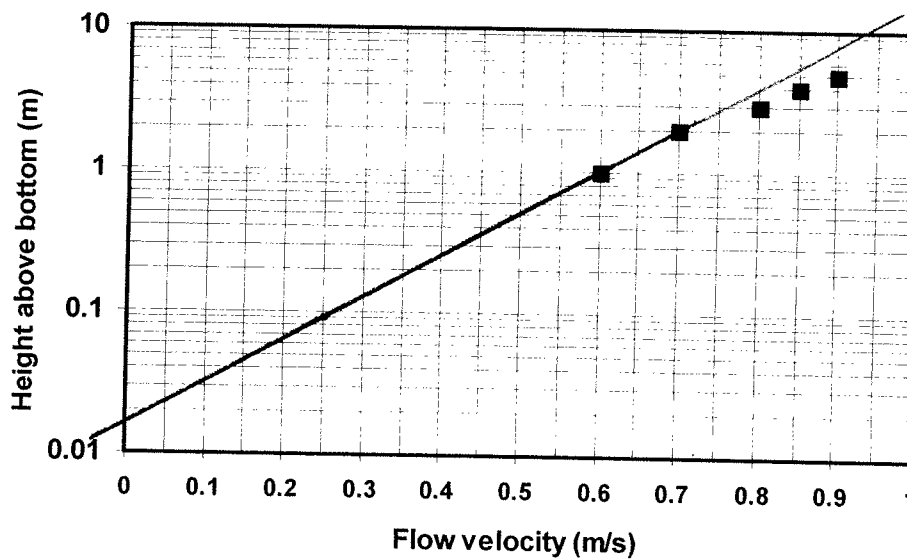


- compute the discharge Q if $k_s = 0.1 \text{ m}$; what is hydraulic roughness regime?
- compute the discharge Q if $k_s = 0.0001 \text{ m}$; what is hydraulic roughness regime?
- what is the depth h , if $Q = 200 \text{ m}^3/\text{s}$ and $k_s = 0.1 \text{ m}$

5. Flow velocity measurements have been carried out in a river with water depth $h = 6$ m, as follows:

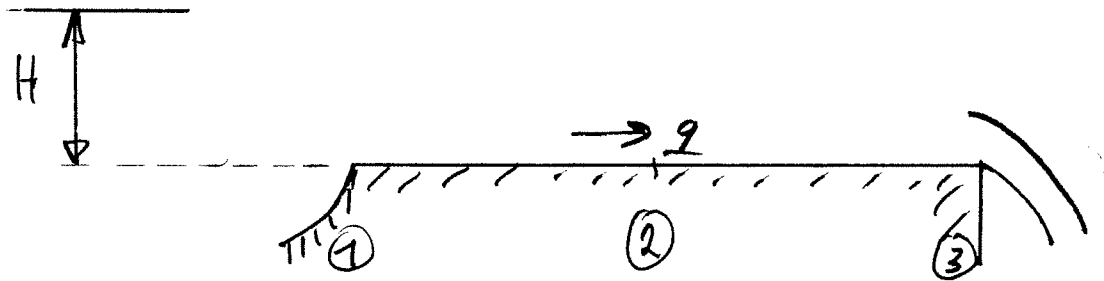
$z_1 = 1$ m	$u_1 = 0.6$ m/s
$z_2 = 2$ m	$u_2 = 0.7$ m/s
$z_3 = 3$ m	$u_3 = 0.8$ m/s
$z_4 = 4$ m	$u_4 = 0.85$ m/s
$z_5 = 5$ m	$u_5 = 0.9$ m/s

viscosity = 0.000001 m²/s, kappa $\kappa = 0.4$



- what is the depth-averaged flow velocity and at what height above the bottom is the local velocity equal to the depth-averaged velocity?
- compute the bed-shear velocity u_* if the hydraulic roughness is $k_s = 0.5$ m using the measured velocity in the lowest point (z_1) only?
- compute the bed-shear velocity u_* and the hydraulic roughness k_s using the measured velocities in the lowest two points (z_1 and z_2)
- same, using all points; fit a line through the point (by eye)

6. A deep reservoir is connected to a channel with a horizontal bottom
Free overflow at end of channel
 $q = 0.5 \text{ m}^2/\text{s}$, $g = 10 \text{ m/s}^2$ $H = 0.6 \text{ m}$, $C = 100 \text{ m}^{0.5}/\text{s}$



- a) what is the water depth h_3 at the end of the channel and at the entrance h_1 of the channel?
- b) what is the length of the channel?
- c) what is the water depth h_2 in the middle of the channel?

7. Theoretical questions

- a) Why is the fluid pressure in a uniform river hydrostatic?
- b) When is it allowed to use the Bernoulli equation and when the momentum equation?
- c) What is the difference between the momentum equation of Euler and the Reynolds equations?
- d) What three terms do we have in the Bernoulli equation?
- e) How can a fluid particle describe a curved path? Which type of forces do occur?
- f) The Froude number is the ratio of
The Reynolds number is the ratio of
- g) How many water depth regimes are possible for a given discharge q ?
What is the most essential feature of each regime?
When is there only one water depth possible?
- h) What is a hydraulic jump and when does it occur?
How can you manipulate a hydraulic jump (upstream or downstream)?
The depth upstream is 0.05 m, the velocity upstream is 3 m/s.
The depth downstream is 0.3 m; the velocity downstream is 0.5 m/s.
What is the energy loss (in meters)?
- i) In what type of flow regime does a viscous sublayer occur?
What is the thickness of the viscous sublayer: 0.1, 1 or 10 mm?
- j) What is a static pitot-tube?
What is a dynamic pitot-tube?
- k) The bed of river is covered by grass. How can you determine the effective roughness k_s of it?
- l) The flow velocities near the surface of a river often are smaller than those at lower levels.
What are possible cause for this?
- m) The dimension of the Chezy coefficient is $m^{0.5}/s$. Why?
- n) The Belanger equation can be used to compute the water level changes in a river (upstream of a weir). Which three forces are included?
- o) You are in a boat (length=20 m) on a small river attached to an anchor. The boat does not move. you have a rope, a stone, a watch, a piece of wood and a ruler with a length of 0.5 m.
How can you estimate the discharge q per unit width?

